

ABBREVIATIONS

Table with 3 columns: Abbreviation, Description, and Unit/Quantity. Includes items like ADDL (Additional), AESS (Structurally Exposed Structural Steel), AGGR (Aggregate), etc.

GENERAL NOTES

DIVISION 01 - Section 01 00 00 GENERAL REQUIREMENTS

- 1. The Contractor shall verify all dimensions and conditions prior to starting construction. The Engineer shall be notified of any discrepancies or inconsistencies.
2. Do not scale the Drawings for working dimensions.
3. Notes and details on Drawings shall take precedence over General Notes and Typical Details.
4. All work shall conform to the minimum standards of the following code:
The 2016 edition of the California Building Code, and any other regulating agencies which have authority over any portion of the work, and those codes and standards listed in these notes and Specifications.
5. See Structural Drawings for the following:
a. Size and location of window and door openings.
b. Size and location of concrete curbs, floor drains, and depressed areas.
c. Size and location of floor and roof openings except as shown.
d. Size and location of interior and exterior non-bearing partitions.
6. See Mechanical, Plumbing, and Electrical Drawings for the following:
a. Electrical conduit runs, boxes, and outlets in walls, size and location of equipment bases.
b. Pipe runs, sleeves, hangers, trenches, and openings.
c. Concrete inserts for fixtures.
7. Contractor shall investigate site during clearing and earth work operations for filled excavations or buried structures such as cesspools, cisterns, foundations, etc.
8. The contract Structural Drawings and Specifications represent the finished structure. They do not indicate the method of construction.
9. Openings, pockets, etc. larger than 6 inches shall not be placed in slabs, decks, beams, joists, columns, walls, etc.
10. Construction materials shall be spread out if placed on framed floors or roof.
11. Shop Drawings submitted to the Structural Engineer for review shall consist of the number of sets to be returned plus one.
12. Adhesive anchors shall be HITI HIT HY150 epoxy per ICBO ER-5193 with ASTM A-36 threaded rod or approved equal u.n.o.
13. Design loads:
Dead Load: 15 psf.
Live Load: 303 psf. (snow)
Pedestrian Load: 90 psf
Vehicle Load (Construction): 2500 lbs

AAASHTO Design Requirements:
Wind Design Data:
1. Horizontal (HWL): 29.2 psf
2. Vertical (VWL): 120 plf
Earthquake Design Data:
Seismic Analysis not Required for Simple Span Bridge (AASHTO 4.7.4.2)
PGA = 0.47
Fpga = 1.175
As = 0.552
EL = 0.331xDL = 212 lbs

DIVISION 01 - Section 01 11 00 SUMMARY OF WORK

- 1. It shall be the contractors direct responsibility to comply with typical details and general notes as delineated or defined on the typical detail drawings of these contract documents regardless of specific flagging or reference to applicable note or detail.
2. It shall be the contractor's responsibility to coordinate with all trades regarding utilities passing through and under footings.
3. Top of footing elevations noted are minimum. see note 2 for additional requirements.
4. Contractor to verify and coordinate all locations and sizes of openings in slabs, slab depressions, and curbs for all related construction prior to floor layout or construction.
5. Drawings are diagrammatic in nature and are not intended to indicate every opening or penetration in roof or other structure.
6. Contractor to verify with appropriate sub-contractors the exact location, weight, and intended method of attachment of all items to be suspended from or in any way attached to any roof framing or other structural member.
7. Contractor to verify dimensions with architect prior to construction.
8. Contractor to verify all existing conditions and dimensions and notify the architect in writing of any discrepancies.
9. The contractor and all subcontractors he intends to use (including agents and suppliers) are aware of and acknowledge that close coordination among architectural, mechanical, electrical and structural drawings is required for the following:
a. Determination of all column locations and sizes.
b. Determination of top of floor, top of steel, wall plate and top of beam elevations.
c. Verification of all dimensions.
d. Verify all tops of footings.
10. The contractor and all subcontractors he intends to use (including agents and suppliers) shall make consideration for and include Monies for the above in preparation of their bids. This requirement shall supersede any contained in the AISC "Manual of Steel Construction".

DIVISION 01 - Section 01 45 00 SPECIAL INSPECTIONS AND DEFERRED SUBMITTALS

SEE SHEET S0.2 FOR SPECIAL INSPECTION NOTES

DIVISION 01 - Section 01 82 13 FOUNDATION PERFORMANCE REQUIREMENTS

- 1. Foundation design based on soils report by the following company:
H&K an NVS Company
10775 Pioneer Trail, #213
Truckee, CA 96161
(530) 587-5156 Fax: (530) 587-5196
Job No. 42175.03 DATE: 6/1/18
2. Footings are designed based on an allowable soil bearing pressure of 2500 psf with 1/3 increase for short-term loads.
3. Contractor shall provide for proper de-watering of excavations from surface water, ground water, seepage, etc.
4. Contractor shall provide for design and installation of all cribbing, sheeting and shoring required to safely and adequately retain the earth banks.
5. Excavations for footings shall be approved by the Soils Engineer prior to placing the concrete and reinforcing.
6. All excavations shall be properly backfilled. Do not place backfill behind retaining walls before concrete has attained full design strength.
7. Footings shall be placed and estimated according to depths shown on Drawings.
8. Footing backfill and utility trench backfill within building area shall be mechanically compacted in layers, to the approval of the Soils Engineer.
9. All abandoned footings, utilities, etc., that interfere with new construction shall be removed.

DIVISION 03 - Section 03 00 00 CONCRETE

- 1. All phases of work pertaining to the concrete construction shall conform to the "Building Code Requirements for Reinforced Concrete" (ACI 318) and the "Specifications for Structural Concrete for Buildings" (ACI 301) latest approved editions, with modifications as noted in the Drawings or Specifications.
2. Reinforced concrete design is by the "Ultimate Strength Design method".
3. Concrete mixes shall be designed by a qualified testing laboratory and approved by the Structural Engineer.
4. Schedule of Structural concrete 28-day strengths and types:
LOCATION IN STRUCTURE STRENGTH PSI TYPE
Footings: 3500 Normal Wt. 145 ± 5 pc
Concrete retaining walls: 4000 Normal Wt. 145 ± 5 pc
5. Portland cement shall conform to ASTM C-150, type II. Use minimum 6 sacks cement/c-y and maximum 3" slump with water (slump may be increased with admixtures that do not promote shrinkage).
6. Maximum aggregate size shall conform with the following: 1/5 distance between forms, 3/4 distance between reinforcing bars, 1/3 thickness of slab.
7. Forms for elevated concrete beams shall be laid out and constructed to provide the specified cambers shown on the Drawings.
8. Dry pack under base plates, sill plates, etc., see Specifications.
9. Concrete mixing operations, etc., shall conform to ASTM C-94.
10. Placement of concrete shall conform to ACI-318 requirements.
11. If columns and walls are placed with floor, two hours must elapse between end of column or wall pour and beginning of floor pour.
12. Clear coverage of concrete over other reinforcing bars shall be as follows:
a. Concrete poured directly against earth, 3 in. clear to reinforcing.
b. Structural slabs: 1 in. clear (top to bottom).
c. Formed concrete with earth backing: 2 in. clear.
d. Slabs on Grade: center in slab.
13. All reinforcing bars, anchor bolts and other concrete inserts shall be well secured in position prior to placing concrete.
14. Provide sleeves for plumbing and electrical openings in concrete before placing. Do not cut any reinforcing which may conflict.
15. Conduit shall not be placed in slabs or walls unless specifically detailed otherwise.
16. Projecting corners of beams, walls, columns, etc., shall be formed with a 1/2 in. chamfer unless otherwise noted on Architectural Drawings.
17. Curing compounds used on concrete that is to receive a resilient tile finish shall be approved by the tile manufacturer before use.
18. Place and protect concrete in compliance with ACI 305 and 306, respectively, during hot and cold exposure conditions.

DIVISION 03 - Section 03 21 00 REINFORCING STEEL

- 1. All reinforcing steel shall be detailed and placed in conformance with the "Building Code Requirements for Reinforced Concrete" (ACI 318 latest approved edition), and the "Manual of Standard Practice for Reinforced Concrete Construction" (latest edition) by the C.R.S.I. and the W.C.R.S.I., as modified by the project Drawings and Specifications.
2. Deformed reinforcing bars shall be ASTM A-615 Grade 60 except ties, stirrups, slab dowels and reinforcing bars in non structural concrete such as slabs on grade, which may be Grade 40, unless noted otherwise.
3. Welding of reinforcing shall be with low hydrogen electrodes in conformance with "Recommended Practices for Welding Reinforcing Steel, etc.", American Welding Society, AWS-D1.4. See Specifications.
4. All reinforcing bar bends shall be made cold.
5. Welded wire fabric shall conform to ASTM A-185.
6. Minimum lap of welded wire fabric shall be 6 inches or one full mesh and one half, which ever is greater.
7. Reinforcing splices shall be made only where indicated on the drawings.
8. Dowels between footings and walls or columns shall be the same grade, size and spacing or number as the vertical reinforcing, respectively.
9. All bars shall be marked so their identification can be made when the final in-place inspection is made.
10. Splice reinforcing per detail 5/S0.2 for both concrete and masonry. Splice all reinforcing bars 2'-0" minimum.
11. All reinforcing bars to be tied in place before pouring concrete or grout.
12. Do not splice reinforcing steel in middle third of walls.

DIVISION 06 - Section 06 11 00 WOOD FRAMING

- 1. Framing lumber shall be Alaskan Yellow Cedar 20F-V12 or better, with moisture content < 19% unless otherwise noted.
2. All bolts shall conform to ASTM A-307. Bolt holes shall be 1/16 in. maximum larger than the bolt size.
3. Standard cut washers shall be used under bolt heads and nuts against wood.
4. Do not notch joists, rafters or beams, except where shown in details.
5. Nailed connections shall conform to the minimum nailing schedule of table 2304.9.1 of the California Building Code.
6. Unless noted otherwise, pre-manufactured framing connectors called for on the Drawings shall be Simpson Strong-Tie connectors, or approved equal.
7. All GluLam Beams specified shall have the following minimum design strengths:
width and depth as shown on plan: fb=2000 psi, fv=265 psi, e=1,500,000
8. Do NOT notch beams, joists, and studs, (U.N.O.)

DIVISION 06 - Section 06 18 00 GLUED-LAMINATED CONSTRUCTION

- 1. General: Provide structural glued-laminated timber that complies with AITC/ANSI A190.1 or research/evaluation reports acceptable to authorities having jurisdiction.
2. Glued laminated wood beams shall be Alaskan Yellow Cedar or approved natural decay resistant, combination 20F-V12 AC/AC for simple spans and 20F-V13 AC/AC for cantilevered spans with E=1.5x10^6 psi, all with exterior glue.
3. All laminations shall be parallel to the bottom edge of the beam, unless noted otherwise.
4. For additional requirements see Specifications.
5. "Glu-Lam" beam sizes shown on plans are net sizes. Provide additional laminations as required for shaped ridges, hips, valley members, beam seats, etc.
6. Manufacture "Glu-Lam" as required for moisture content, used in arid condition.
7. Simpson hardware shall be Zmax Hot Dipped Galvanized.

DIVISION 31 - Section 31 62 16 HELICAL PIER

- 1. Hot dip galvanized per ASTM A153-(latest revision).
2. Lead and extension section lengths and helix spacings are nominal.
3. Nominal spacing between helix plates is three times the diameter of the lower helix.
4. Shaft material-hot rolled round-cornered-square (rcs) solid steel bars per ASTM A29J minimum yield strength=90 ksi.
5. Helix material-hot rolled low alloy steel sheet, strip, or plate per ASTM A656, or A1018 grade 80. Minimum yield strength=80 ksi.
6. Coupling bolts: 7/8" diameter x 3-1/2" long hex head per ASTM A193 grade B7.
7. Manufacturer to have in effect utility recognized written quality control for all materials and manufacturing processes.
8. All welding to be done by welders certified under section 5 of the AWS code D1.1.
9. See ICC evaluation service inc., evaluation report NO., ESR-2794 for nominal, design, and allowable strength values and/or conditions of use concerning information presented on this drawing.

Per H&K Soils Report:
Boardwalk Helical Pier Support - Chance Helical Pier or equivalent:

Table with 2 columns: Depth of Pier (feet bgs) and Number of Helices. Values: 10, 2, 6.5/14', 10/12', 10±4'

- Chance SS175 or larger diameter shaft
-Helical Pier material should be suitable for placement in wet environment
-Torque strength/rating=10,500 ft-lb
-Ultimate capacity (compression)=105 kip
-Le based on a torque factor (kt)-10
-Per icc-es ac308 section 3132
-Nominal tension strength (coupling bolt)=100 kip

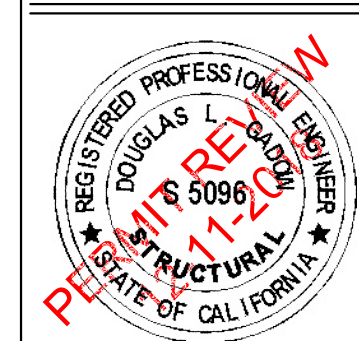
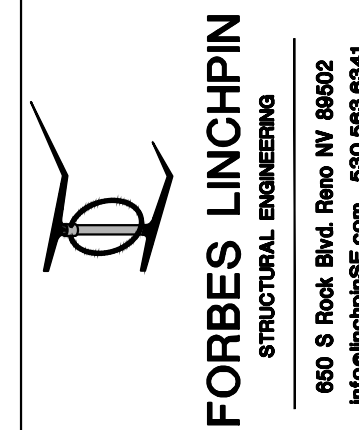
BEARING PAD

Provide elastomeric bearing pad at girder bearing locations to accommodate thermal expansion. Plain elastomer bearing pads and laminated steel bearing pads shall conform to the applicable requirements of ASTM D4014. Laminated fabric bearing pads shall conform to the applicable requirements of AASHTO M251.

Steel reinforced elastomeric bearing pads material requirements include:
- Steel reinforced elastomeric bearing pad (grade 4)
- Durometer hardness (shore a) of 60
- Shear modulus at 7.3f of 175 psi

SHEET INDEX

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S2.0 BOARDWALK SITE PLAN
S2.1 BOARDWALK ENLARGED SECTION "A"
S2.2 BOARDWALK ENLARGED SECTION "B"
S2.3 BOARDWALK ENLARGED SECTION "C"
S2.4 BOARDWALK ENLARGED SECTION "D"
S2.5 BOARDWALK SECTIONS/PARTIAL FRAMING PLAN



REVISIONS

These Drawings have been prepared by Forbes Linchpin Structural Engineering Inc. They are not suitable for use on other projects...

TAHOE DONNER NATURE TRAIL NATURE LOOP SOUTH TRAIL TAHOE DONNER TRUCKEE, CA

DESIGNED BY D.G.
DRAFTED BY T.E.S.

CLIENT INFORMATION
TAHOE DONNER ASSOCIATION 11509 NORTHWOODS BLVD TRUCKEE, CA 96161

PROJECT# 2079

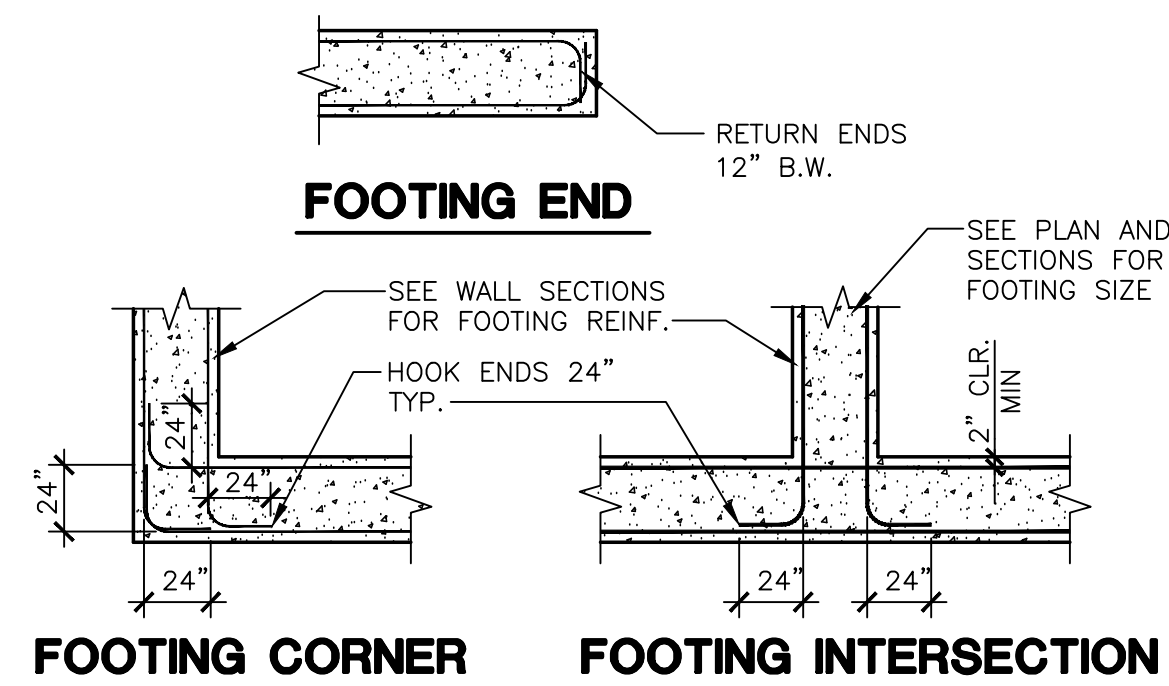
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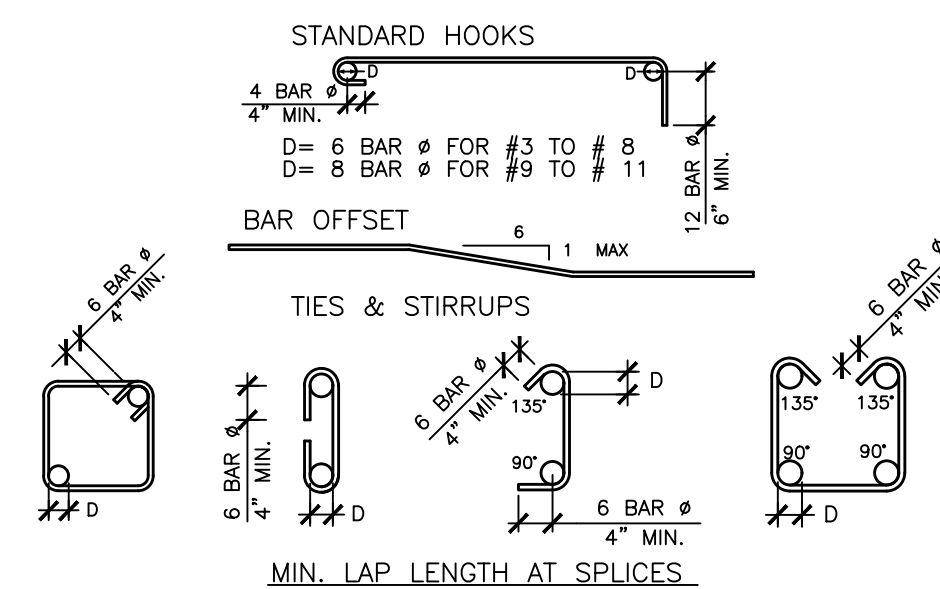
GENERAL NOTES

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PRINT DATE: 02/12/18



2 TYPICAL REINFORCED LAP IN FOOTINGS
C-000-107/N.T.S.



SIZE OF REINF.	#3	#4	#5	#6	#7	#8	#9	#10
CONCRETE	24"	30"	46"	65"	89"	117"	148"	188"
CMU - SINGLE CURTAIN OF REINFORCEMENT								
TYPICAL	24"	24"	30"	36"	42"	48"	-	-
w/ SPLICE AT MID SPAN / HEIGHT	27"	36"	45"	54"	63"	72"	-	-
CMU - DOUBLE CURTAIN OF REINFORCEMENT								
TYPICAL	24"	31"	39"	47"	55"	63"	-	-
w/ SPLICE AT MID SPAN / HEIGHT	35"	47"	59"	70"	82"	94"	-	-

1 REBAR CONFIG. AND LAPS
C-000-104/N.T.S.

SPECIAL INSPECTION SCHEDULE			
TYPE	CONTINUOUS INSPECTION	PERIODIC INSPECTION	REQUIRED OF THIS PROJECT
CONCRETE			
TAKING OF SPECIMENS AND PLACEMENT OF CONCRETE WHERE CALCULATED DESIGN f_c GREATER THAN 2500 psi.		✓	✓
DURING PLACEMENT OF REINFORCING STEEL		✓	✓
STRESSING OF POST-TENSIONING TENDONS			
HIGH STRENGTH GROUT AND MOMENT FRAME ANCHOR ROD PLACEMENT			
INSTALLATION OF DAYTON SUPERIOR BARLOCK REBAR COUPLERS			
ALL EPOXY-SET ANCHORS			
STEEL			
WELDING			
COMPLETE JOINT PENETRATION WELDS			
PERIODIC INSPECTION OF FILLET WELDS			
WELDED STUDS			
FLOOR AND ROOF DECK WELDING			
HIGH-STRENGTH BOLTING (SEE NOTES, SECTION 5)			
STRUCTURAL MASONRY			
DURING PREPARATION AND TAKING OF ANY REQUIRED TEST SPECIMENS			
AT START OF LAYING UNITS, AFTER PLACEMENT OF REINFORCING STEEL, GROUT SPACE PRIOR TO EACH GROUTING OPERATION AND DURING ALL GROUTING OPERATION			
SIMPSON TITEN HD ANCHORS			
AFTER INSTALLATION OF ALL SHEAR WALL AND DIAPHRAGM SHEATHING WITH NAIL SPACING OF 4" o.c. OR LESS			
AFTER INSTALLATION OF A NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS INCLUDING DRAG STRUTS, STRAPS, BRACES AND HOLDOWNS			
HELICAL PIERS	✓		✓

* SPECIAL INSPECTION NEED NOT BE PRESENT CONTINUOUSLY DURING PLACING OF REINFORCING STEEL PROVIDED THE INSPECTOR HAS INSPECTED FOR CONFORMANCE TO PLANS PRIOR TO CLOSING OF FORMS OR DELIVERY OF CONCRETE TO THE JOB SITE.

NOTES

- WELDING (PER CBC 2016, SECTION 17043.M AWS D1.1 AND AISC 341-10) SPECIAL INSPECTION SHALL BE IN ACCORDANCE WITH CBC SECTION 17043 AND AWS D1.1 CHAPTER 6.
 - PERIODIC INSPECTION OF ALL FILLET WELDS OF ALL FIELD WELDING
 - COMPLETE PENETRATION WELDS
 - CONTINUOUS INSPECTION REQUIRED FOR ALL FIELD AND SHOP COMPLETE PENETRATION WELDS.
 - NON-DISTRUCTIVE TESTING REQUIRED FOR ALL COMPLETE PENETRATION WELDS.
- REINFORCING STEEL
 - CONTINUOUS INSPECTION PER CBC AWS D1.4, ACI 318-10 352, ACI 530-10 2.1.10.72 AND 333.4 (B) OF ALL WELD REINFORCING STEEL FOR CONCRETE MOMENT FRAMES AND MASONRY SPLICES. ALL REINFORCING STEEL TO BE WELDED SHALL BE A706.
 - PERIODIC INSPECTION PER CBC AWS D1.4 ACI 318-10 352. OF ALL OTHER WELDING OF REINFORCING STEEL NOT IN ITEM 1 ABOVE ALL REINFORCING STEEL TO BE WELDED SHALL BE A706.
- CONCRETE (PER CBC 2016, SECTION 19015 AND ACI 318-10) SPECIAL INSPECTION SHALL BE IN ACCORDANCE WITH CBC SECTION 1704.4 AND TABLE 1704.4
 - SPECIAL INSPECTION ON CONCRETE PLACED FOR ELEVATED SLAB AT SECOND FLOOR
 - SPECIAL INSPECTION ON FOOTINGS AND SLAB -ONOGRADE IS NOT REQUIRED.
- STRUCTURAL WOOD FOR SEISMIC RESISTANCE (PER CBC 2016, SECTION 1707.3) PERIODIC SPECIAL INSPECTION FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENER COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING DRAG STRUTS, BRACES AND HOLDOWNS.

EXCEPTION FASTING OF WOOD SHEATHING USED FOR WOOD SHEAR WALLS, SHEAR PANELS AND DIAPHRAGMS WHERE THE FASTENER SPACING IS GREATER THAN 4" o.c.
- HIGH-STRENGTH BOLTING (PER CBC 2016, SECTION 1704.3.3 AND AISC 360, SECTION M2.5) WHILE THE WORK IS IN PROGRESS, THE SPECIAL INSPECTOR SHALL DETERMINE THAT THE REQUIREMENTS FOR BOLTS, NUTS, WASHERS AND PAINT, BOLT PARTS AND INSTALLATION AND TIGHTENING IN SUCH STANDARDS ARE MET. FOR BOLTS REQUIRING PRE-TENSIONING, THE SPECIAL INSPECTOR SHALL OBSERVE THE PRE-INSTALLATION TESTING AND CALIBRATION PROCEDURES WHEN SUCH PROCEDURES ARE REQUIRED BY THE INSTALLATION METHOD OR BY PROJECT PLANS OR SPECIFICATION. DETERMINE THAT ALL PILES OF CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED AND MONITOR THE INSTALLATION OF BOLT TO VERIFY THAT THE SELECTED PROCEDURE FOR INSTALLATION IS PROPERLY USED TO TIGHTEN BOLTS. FOR JOINTS REQUIRED TO BE TIGHTENED ONLY TO THE SNUG-TIGHT CONDITION, THE SPECIAL INSPECTOR NEED ONLY VERIFY THAT THE CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED

PERIODIC MONITORING MONITORING OF BOLTS INSTALLATION FOR PRE-TENSIONING IS PERMITTED TO BE PERFORMED ON PERIODIC BASIS WHEN USING THE TURN-OFF-NUT METHOD WITH MATCH-MARKING TECHNIQUES, THE DIRECT TENSION INDICATOR METHOD OR THE ALTERNATE DESIGN FASTENER (TWIST-OFF BOLTS) METHOD. JOINTS DESIGNATED AS SNUG TIGHT NEED BE INSPECTED ONLY ON PERIODIC BASIS.

CONTINUOUS MONITORING MONITORING OF BOLT INSTALLATION FOR PRE-TENSIONING USING THE CALIBRATED WRENCH METHOD OR TURN-OFF-NUT METHOD WITHOUT MATCH-MARKING SHALL BE PERFORMED ON CONTINUOUS BASIS.

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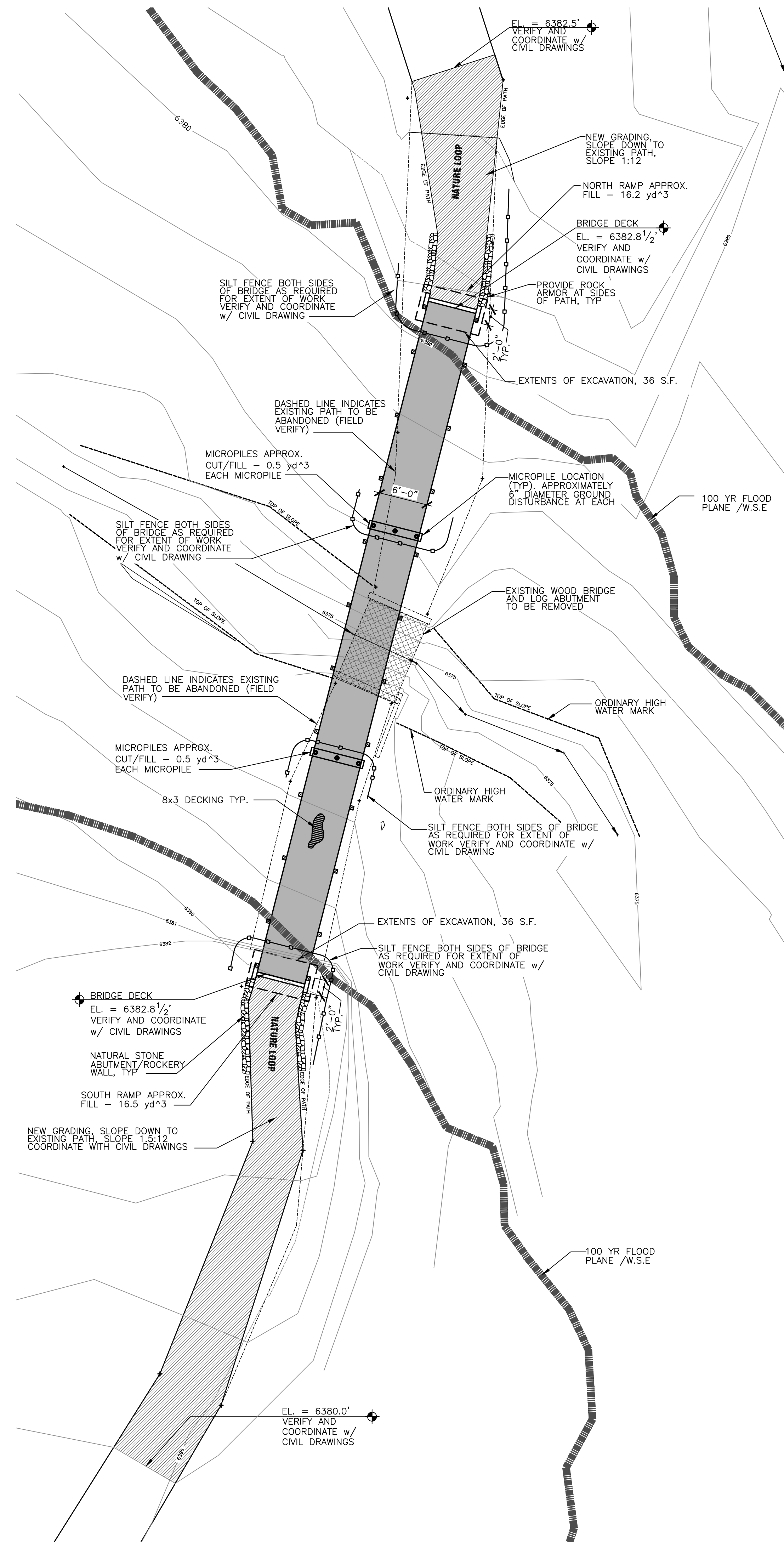
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SCALE AS NOTED

GENERAL NOTES/
TYPICAL DETAILS

S0.2

PRINT DATE: 12/10/18



- ### LEGEND
- SHADED HATCH INDICATE NEW BRIDGE
 - DASHED LINE INDICATES EXISTING PATH TO BE ABANDONED (FIELD VERIFY)
 - 100 YR FLOOD PLANE /W.S.E
 - SILT FENCE AS REQUIRED FOR EXTENT OF WORK VERIFY AND COORDINATE LOCATION w/ CIVIL DRAWING

- ### NOTES
1. CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND COORDINATE AND VERIFY ALL DIMENSIONS WITH THE DRAWINGS. CONTACT ENGINEER WITH DISCREPANCIES BEFORE CONSTRUCTION OCCURS.
 2. THE 100 YEAR FLOOD PLANE WATER SURFACE ELEVATION (WSE) IN THE AREA OF BRIDGE IS 6379'-6"
 3. FREEBOARD ELEVATION ABOVE WSE TO BOTTOM OF STRUCTURE 1'-0"
 4. MAXIMUM ELEVATION ABOVE GROUND SURFACE TO TOP OF WALKING SURFACE: 7'-8 1/2"

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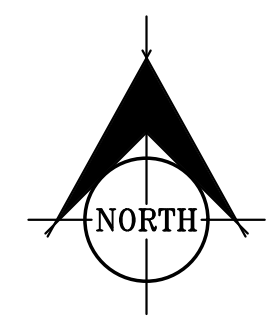
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BRIDGE
 SITE PLANS

S1.1
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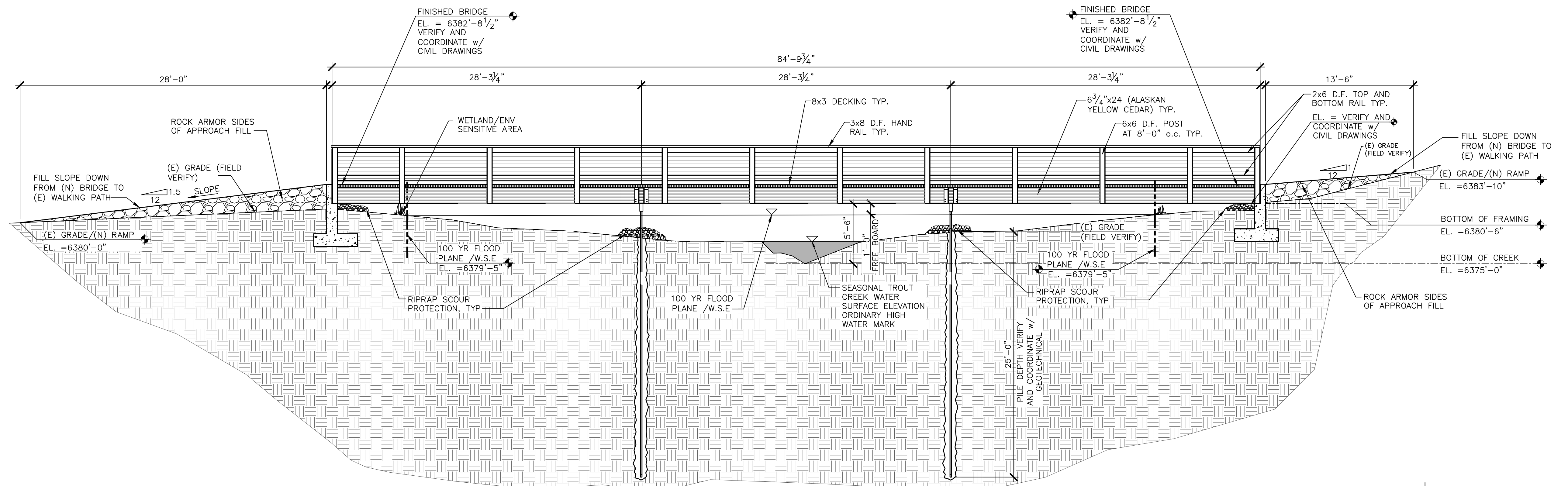
BRIDGE SITE PLAN
 1"=30'-0"



PRINT DATE: 12/10/18



REVISIONS



NOTES

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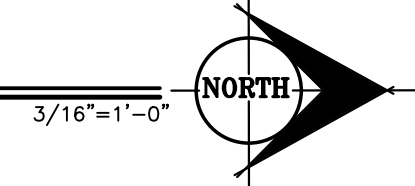
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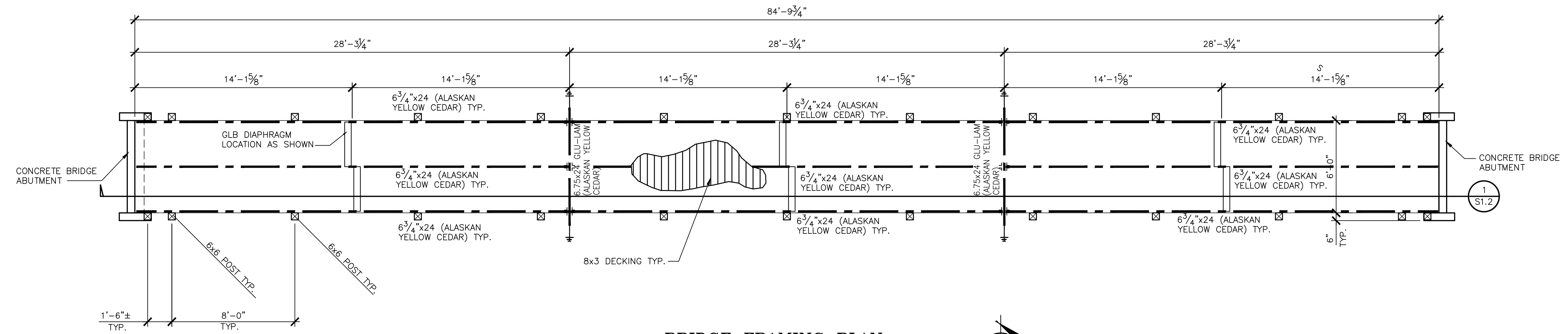
BRIDGE SECTION

**1 BRIDGE SECTION
(LONGITUDINAL)**



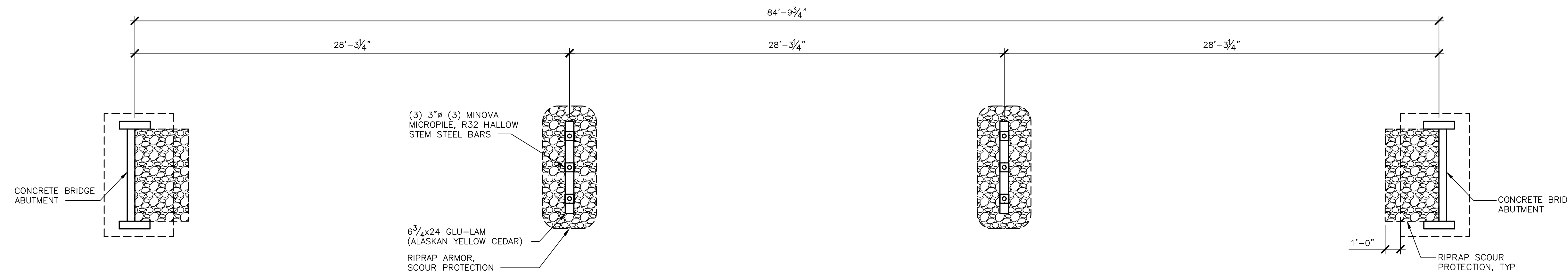
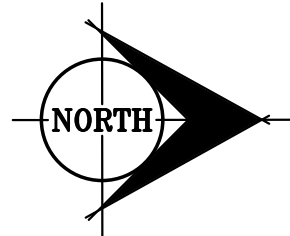


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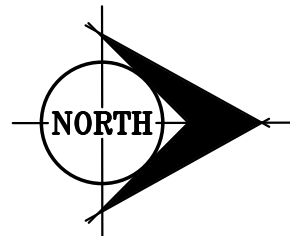
BRIDGE FRAMING PLAN

1/4"=1'-0"



BRIDGE FOUNDATION PLAN

1/4"=1'-0"



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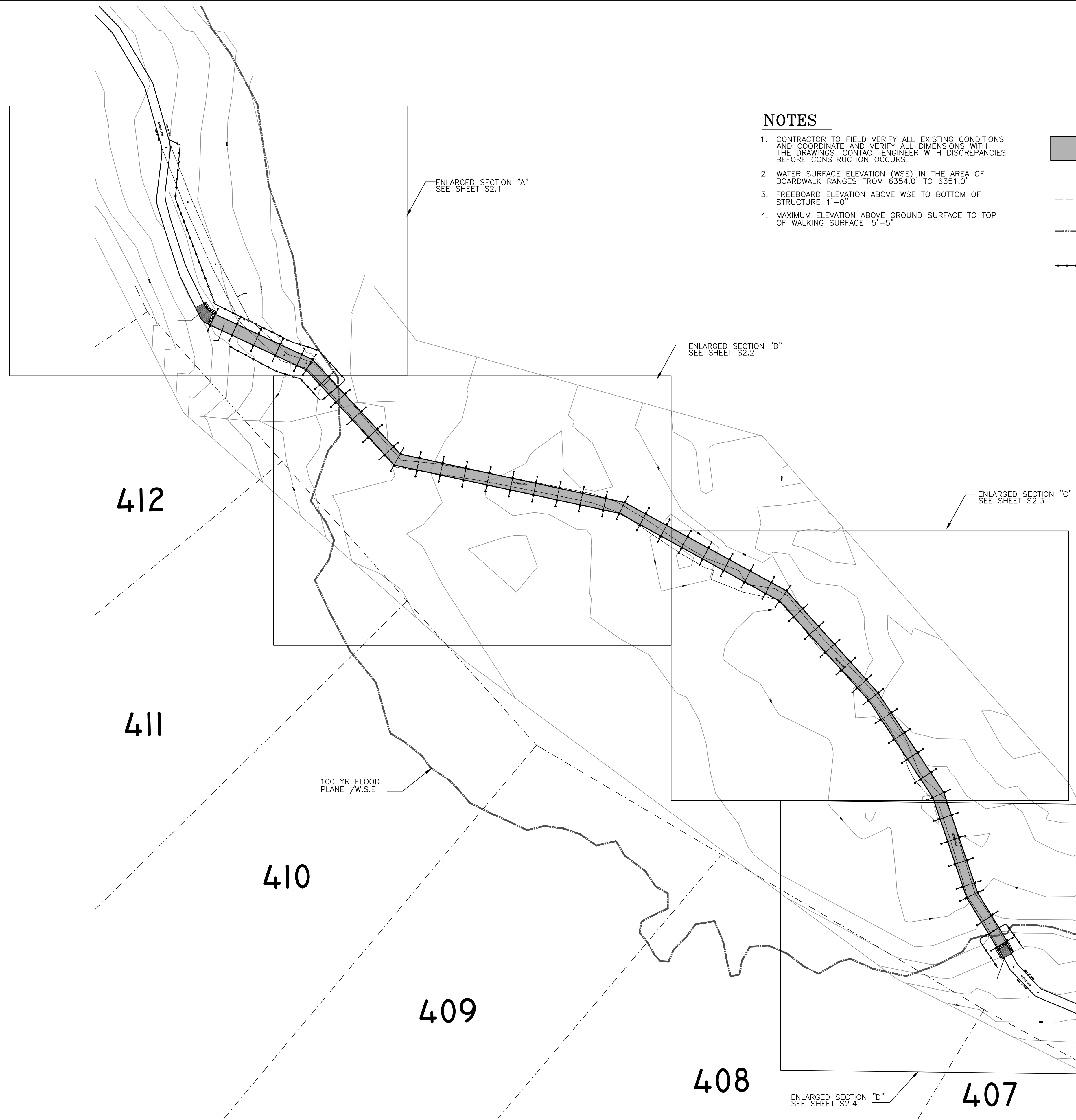
ISSUE DATE 12/10/18

SCALE AS NOTED

BRIDGE FOUNDATION/
FRAMING PLAN

S1.3

PRINT DATE: 12/10/18



NOTES

1. CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND COORDINATE AND VERIFY ALL DIMENSIONS WITH THE DRAWINGS. CONTACT ENGINEER WITH DISCREPANCIES BEFORE CONSTRUCTION OCCURS.
2. WATER SURFACE ELEVATION (WSE) IN THE AREA OF BOARDWALK RANGES FROM 6354.0' TO 6351.0'
3. FREEBOARD ELEVATION ABOVE WSE TO BOTTOM OF STRUCTURE: 1'-0"
4. MAXIMUM ELEVATION ABOVE GROUND SURFACE TO TOP OF WALKING SURFACE: 5'-5"

LEGEND

- SHADED HATCH INDICATE NEW BOARDWALK. EXTENTS DOES NOT CHANGE FROM EXISTING BOARDWALK
- DASHED LINE INDICATES EXISTING PATH/BOARDWALK TO BE ABANDONED (FIELD VERIFY)
- 100 YR FLOOD PLANE /W.S.E
- SILT FENCE AS REQUIRED FOR EXTENT OF WORK VERIFY AND COORDINATE LOCATION w/ CIVIL DRAWING



REVISIONS

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**TAHOE DONNER
 NATURE TRAIL**
 NATURE LOOP
 SOUTH TRAIL
 TAHOE DONNER
 TRUCKEE, CA

DESIGNED BY D.G.
 DRAFTED BY T.E.S.

CLIENT INFORMATION
 TAHOE DONNER
 ASSOCIATION 11509
 NORTHWOODS BLVD
 TRUCKEE, CA 96161

PROJECT# 2079

ISSUE DATE 12/10/18

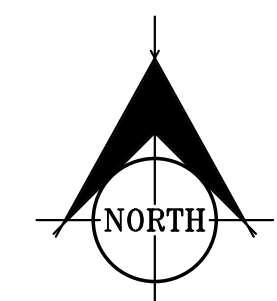
SCALE AS NOTED

BOARDWALK
SITE PLAN

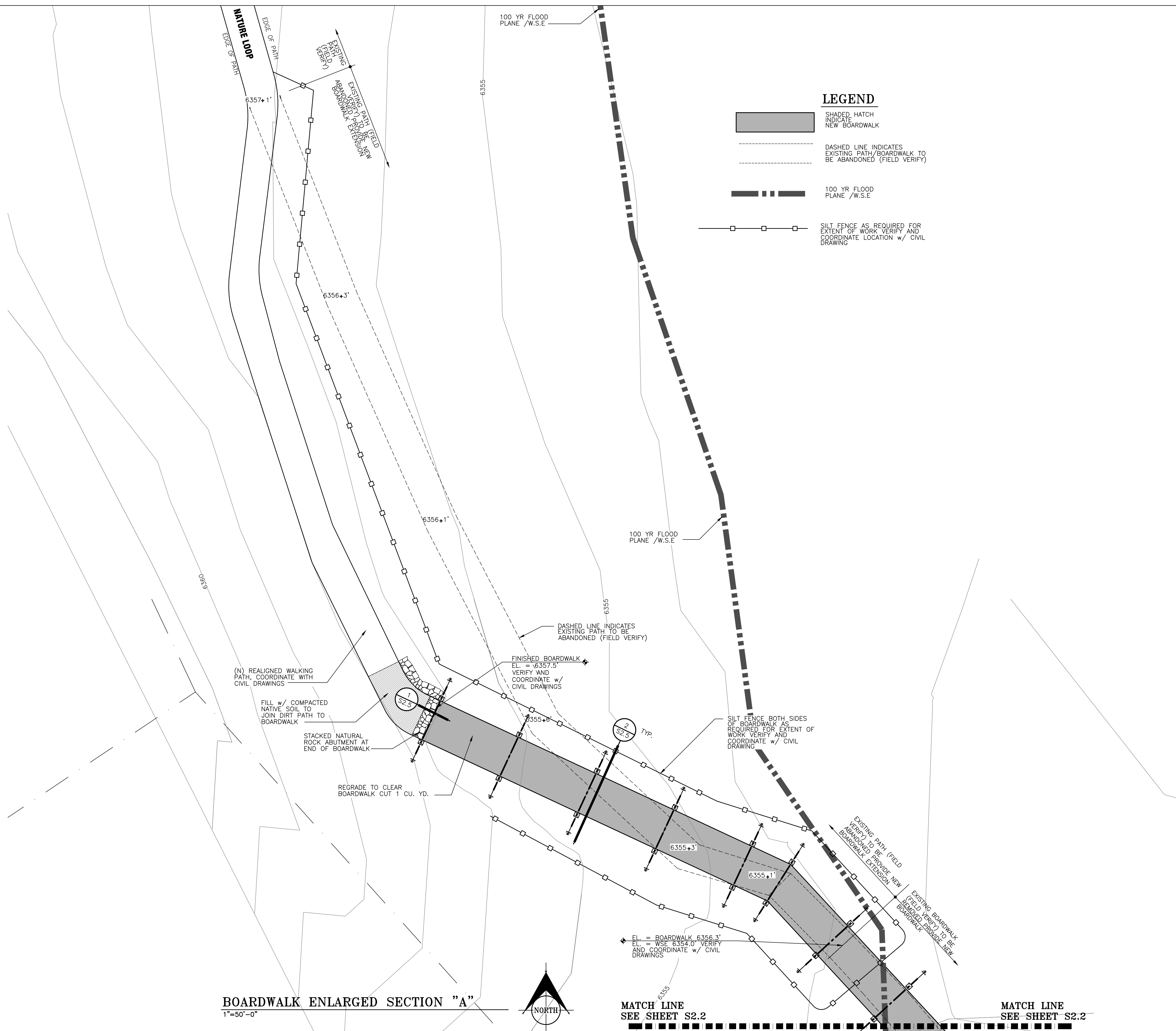
S2.0

15 OF 20


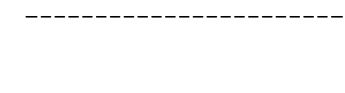

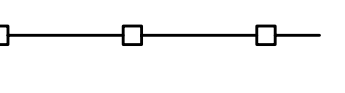
BOARDWALK SITE PLAN
 N.T.S.



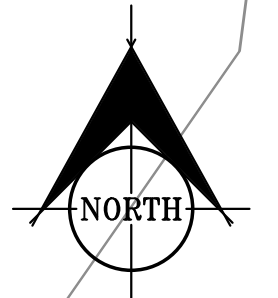
PRINT DATE: 12/10/18



LEGEND

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-  DASHED LINE INDICATES EXISTING PATH/BOARDWALK TO BE ABANDONED (FIELD VERIFY)
-  100 YR FLOOD PLANE /W.S.E
-  SILT FENCE AS REQUIRED FOR EXTENT OF WORK VERIFY AND COORDINATE LOCATION w/ CIVIL DRAWING

BOARDWALK ENLARGED SECTION "A"
1"=50'-0"



MATCH LINE
SEE SHEET S2.2

MATCH LINE
SEE SHEET S2.2



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PROJECT# 2079

ISSUE DATE 12/10/18

SCALE AS NOTED

BOARDWALK ENLARGED SECTION "A"

S2.1

MATCH LINE
SEE SHEET S2.1

MATCH LINE
SEE SHEET S2.1

100 YR FLOOD
PLANE /W.S.E

LEGEND

- SHADED HATCH INDICATE NEW BOARDWALK
- DASHED LINE INDICATES EXISTING PATH/BOARDWALK TO BE ABANDONED (FIELD VERIFY)
- SILT FENCE AS REQUIRED FOR EXTENT OF WORK VERIFY AND COORDINATE LOCATION w/ CIVIL DRAWING

DASHED LINE INDICATES EXISTING BOARDWALK TO BE REMOVED (FIELD VERIFY)

2
S2.5
TYP.

NATURE LOOP

MATCH LINE
SEE SHEET S2.3

MATCH LINE
SEE SHEET S2.3

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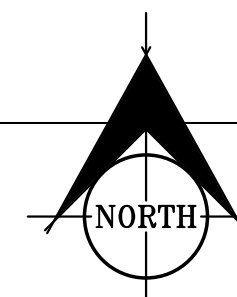
SCALE AS NOTED

BOARDWALK
ENLARGED
SECTION "B"

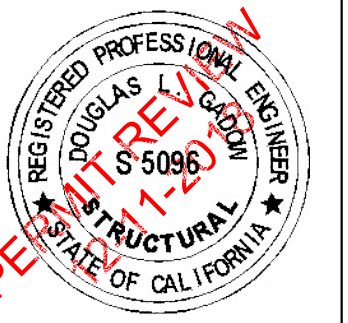
S2.2

17 OF 20

BOARDWALK ENLARGED SECTION "B"
1"=50'-0"



PRINT DATE: 12/10/18



REVISIONS

PRINT DATE: 12/10/18



LEGEND

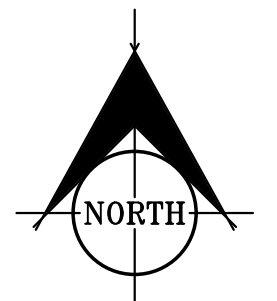
SHADED HATCH INDICATE NEW BOARDWALK

DASHED LINE INDICATES EXISTING PATH/BOARDWALK TO BE ABANDONED (FIELD VERIFY)

MATCH LINE SEE SHEET S2.2

MATCH LINE SEE SHEET S2.2

BOARDWALK ENLARGED SECTION "C"
1"=50'-0"



MATCH LINE SEE SHEET S2.4

MATCH LINE SEE SHEET S2.4



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DRAFTED BY T.E.S.

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TAHOE DONNER ASSOCIATION 11509 NORTHWOODS BLVD TRUCKEE, CA 96161

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ISSUE DATE 12/10/18

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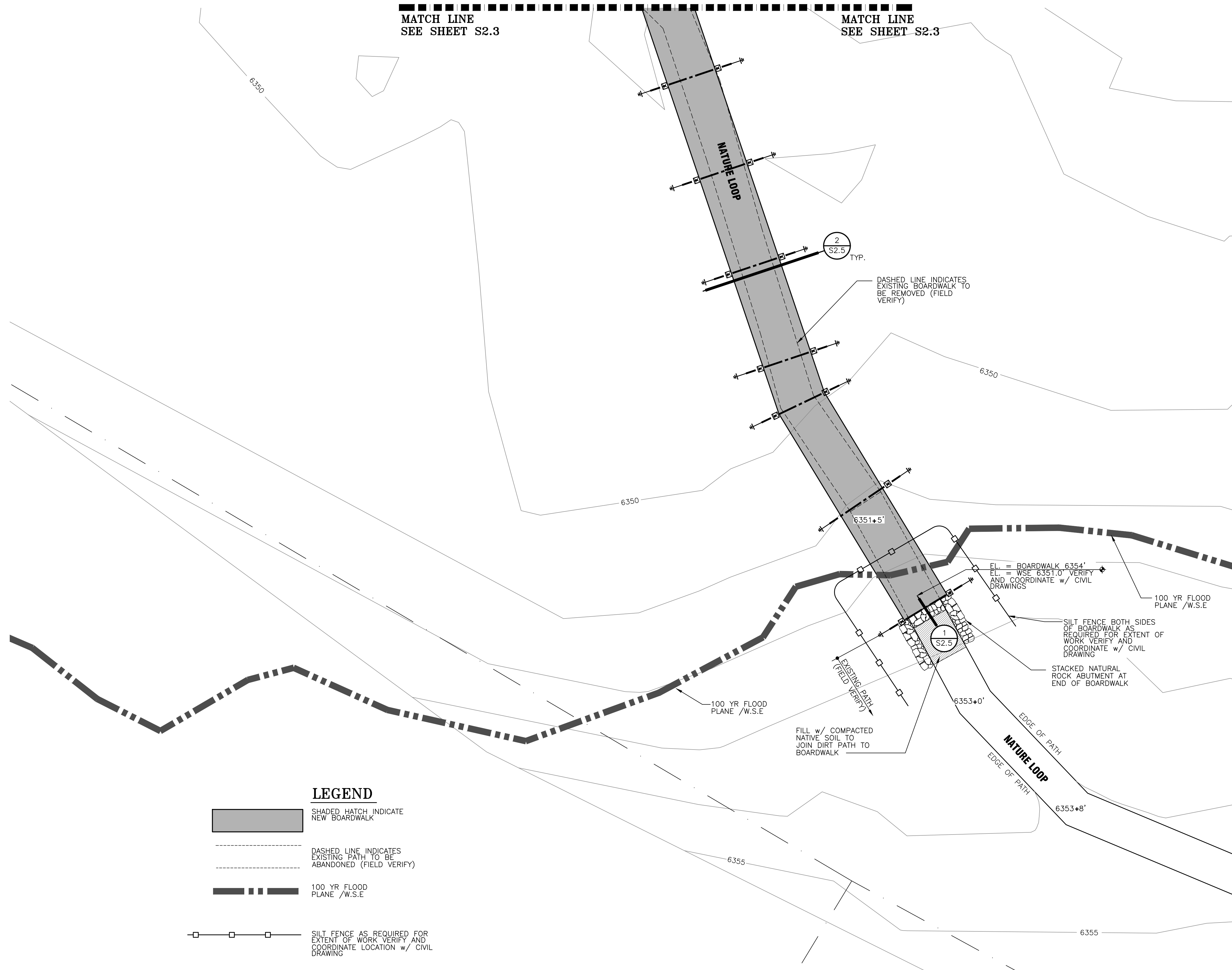
BOARDWALK ENLARGED SECTION "D"

S2.3




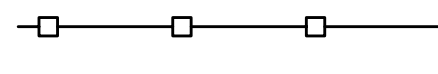
PRINT DATE: 12/10/18

MATCH LINE
SEE SHEET S2.3

MATCH LINE
SEE SHEET S2.3



LEGEND

-  SHADED HATCH INDICATE NEW BOARDWALK
-  DASHED LINE INDICATES EXISTING PATH TO BE ABANDONED (FIELD VERIFY)
-  100 YR FLOOD PLANE /W.S.E.
-  SILT FENCE AS REQUIRED FOR EXTENT OF WORK VERIFY AND COORDINATE LOCATION w/ CIVIL DRAWING

BOARDWALK ENLARGED SECTION "D"
1"=50'-0"



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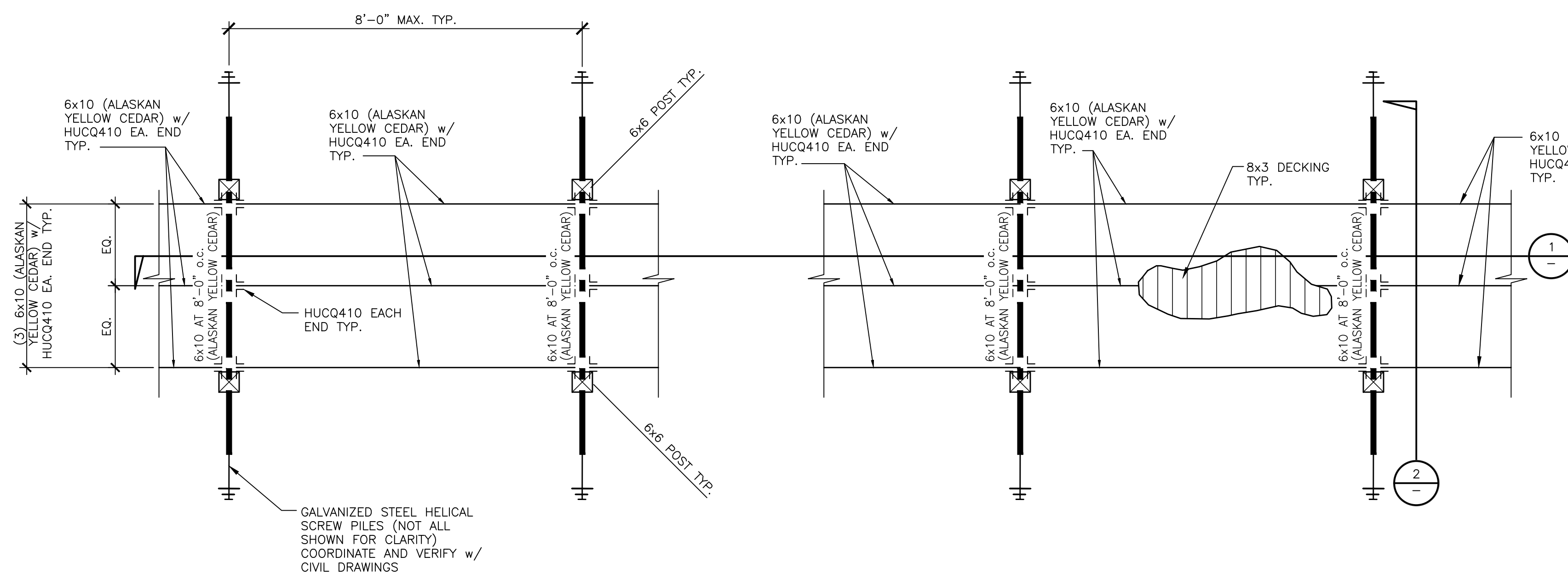
PROJECT# 2079

ISSUE DATE 12/10/18

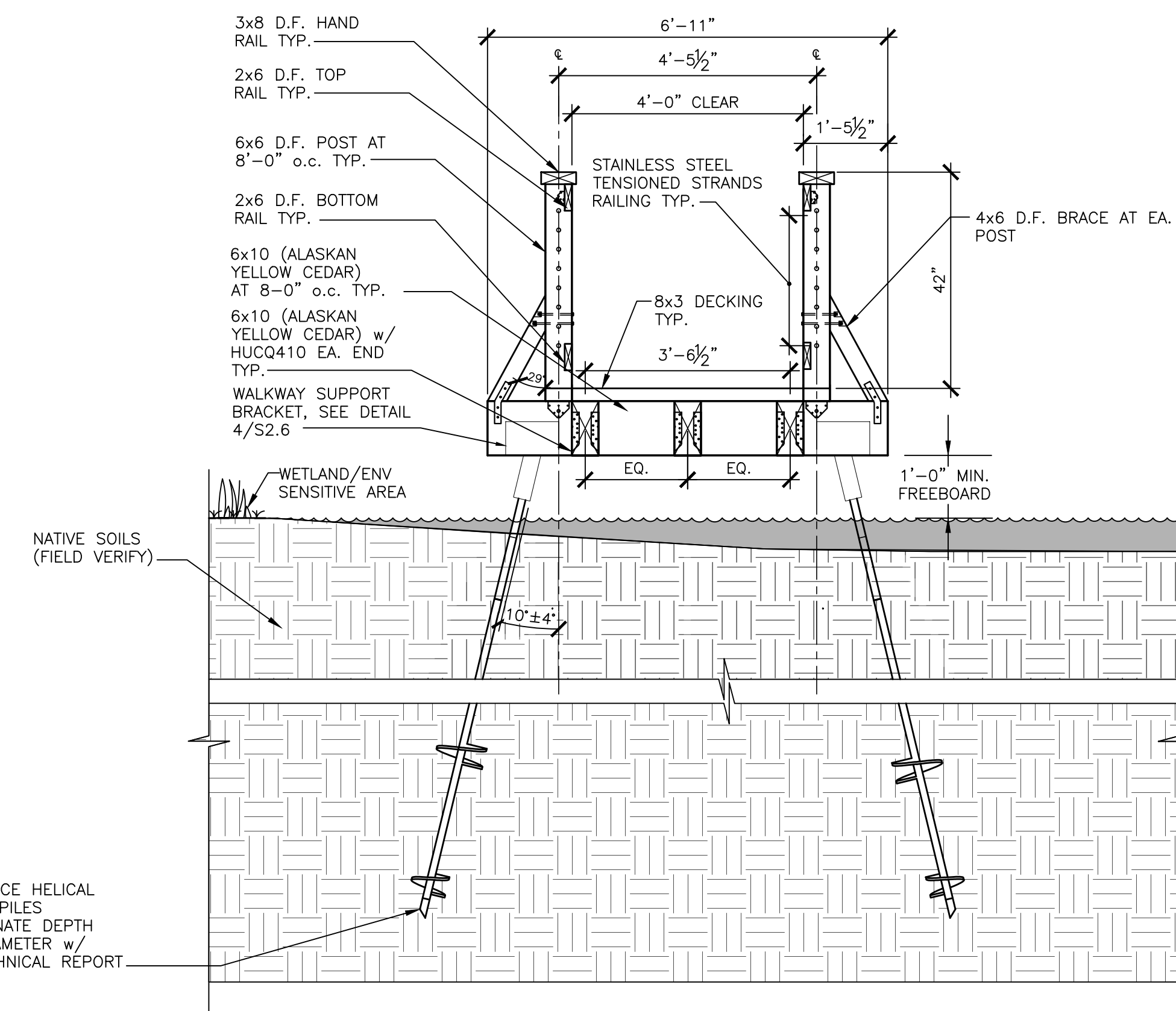
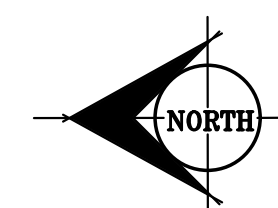
SCALE AS NOTED

**BOARDWALK
ENLARGED
SECTION "D"**

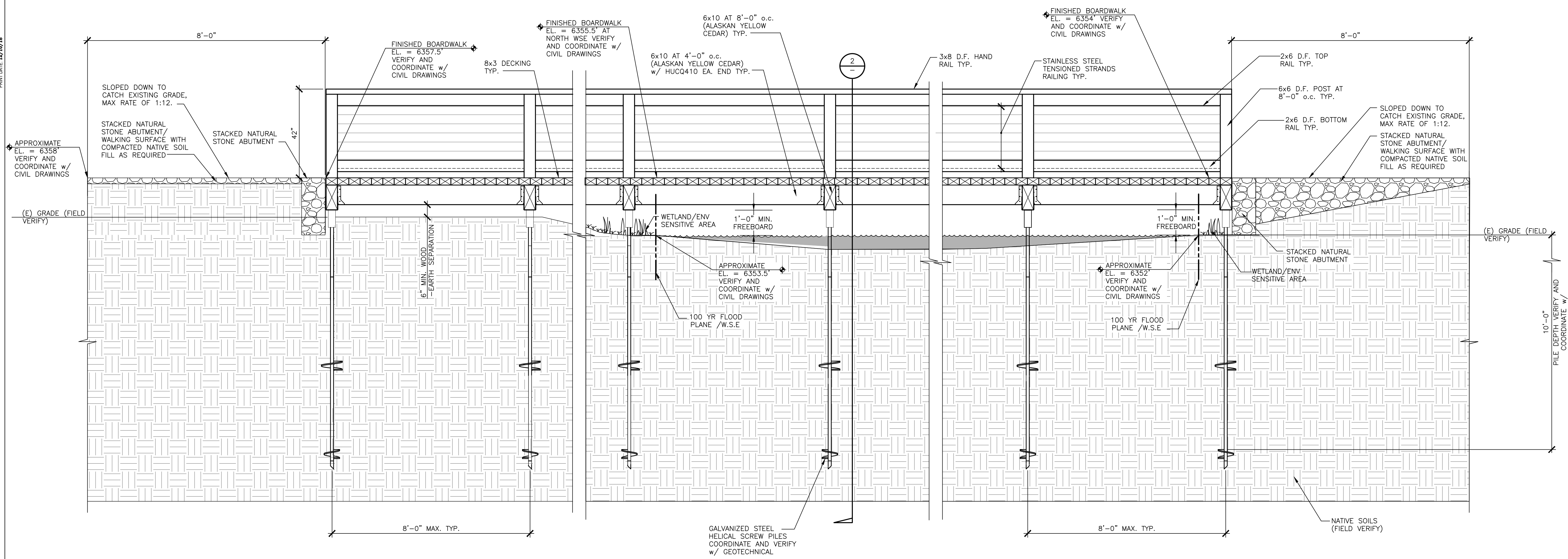
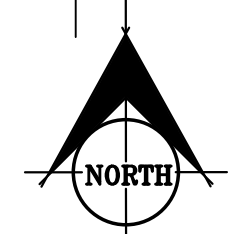
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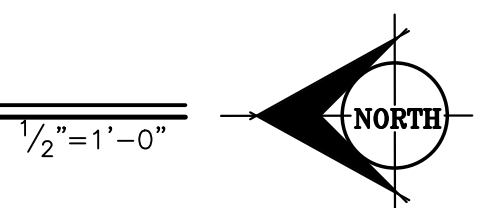
PARTIAL BOARDWALK FRAMING PLAN
1/2"=1'-0"



2 BOARDWALK SECTION (TRANSVERSE)
1/2"=1'-0"



1 BOARDWALK SECTION (LONGITUDINAL)
1/2"=1'-0"



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SCALE AS NOTED

BOARDWALK SECTIONS/PARTIAL FRAMING PLAN